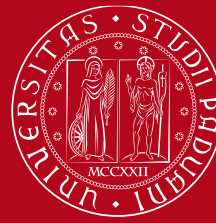




1222 · 2022
800
ANNI



UNIVERSITÀ
DEGLI STUDI
DI PADOVA



DIPARTIMENTO DI INGEGNERIA
CIVILE, EDILE E AMBIENTALE
DEPARTMENT OF CIVIL, ARCHITECTURAL
AND ENVIRONMENTAL ENGINEERING



autoperforanti
SIRIVE®



PROGETTO UNIMPRESA 2019 – TITANO

SISTEMI INNOVATIVI DI MONITORAGGIO GEOTECNICO MEDIANTE SENSORI IN FIBRA OTTICA

**SALA AUDITORIUM DELL'ORTO BOTANICO
28 GIUGNO 2022, ORE 9.00/17.30**

SVILUPPO DELLE FORZE DI STABILIZZAZIONE NEGLI ANCORAGGI PASSIVI **Development of stabilization actions in passive anchors**

Simonetta Cola⁽¹⁾, **Lorenzo Brezzi**⁽¹⁾, Luca Schenato⁽²⁾, Francine Tchamaleo Pangop⁽¹⁾,
Luca Palmieri⁽³⁾, Massimo Lovison⁽⁴⁾, Alberto Bisson⁽⁵⁾

⁽¹⁾ DICEA, Università di Padova

⁽²⁾ CNR-IRPI Padova

⁽³⁾ DEI, Università di Padova

⁽⁴⁾ Provincia di Vicenza

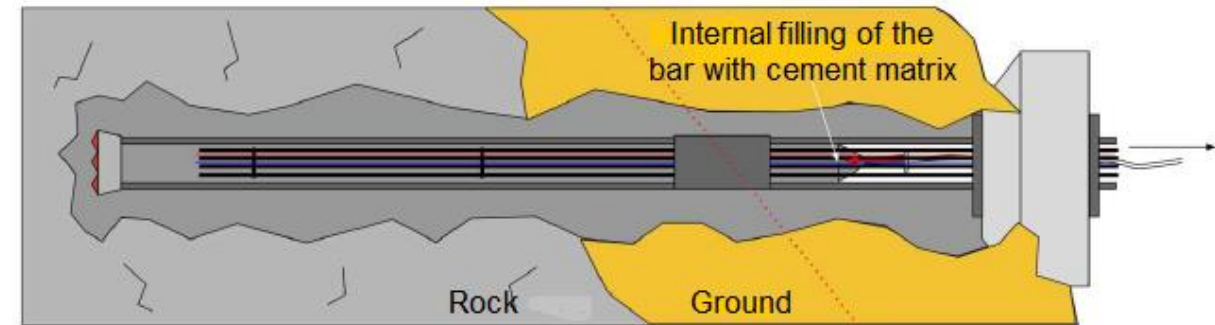
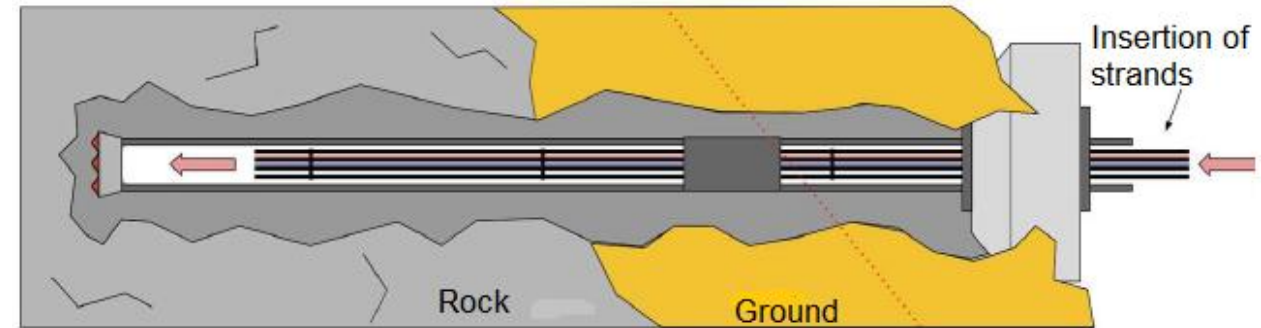
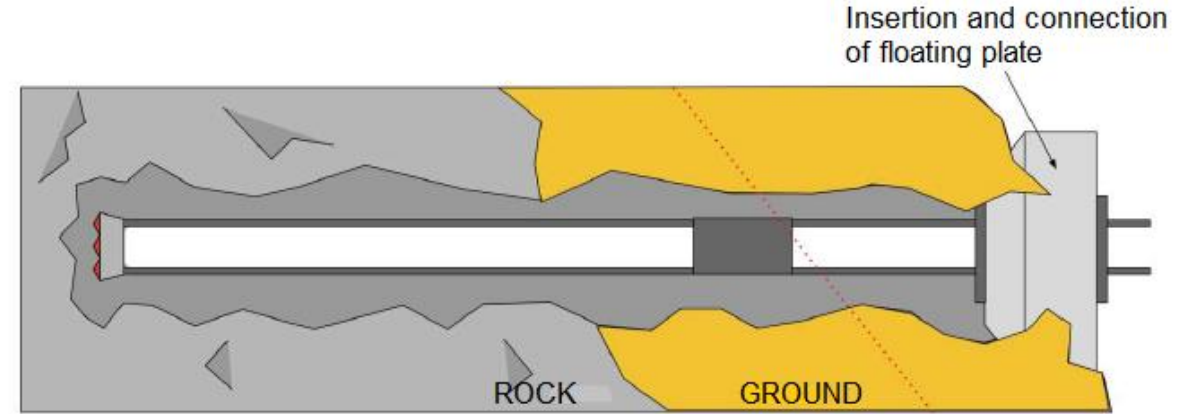
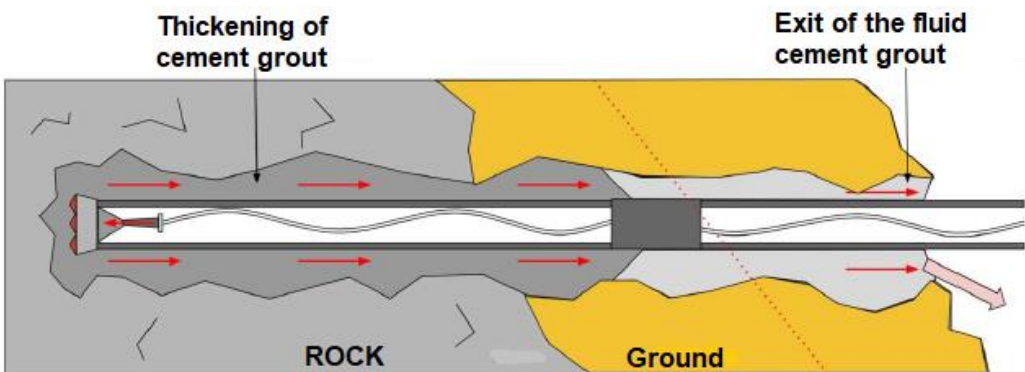
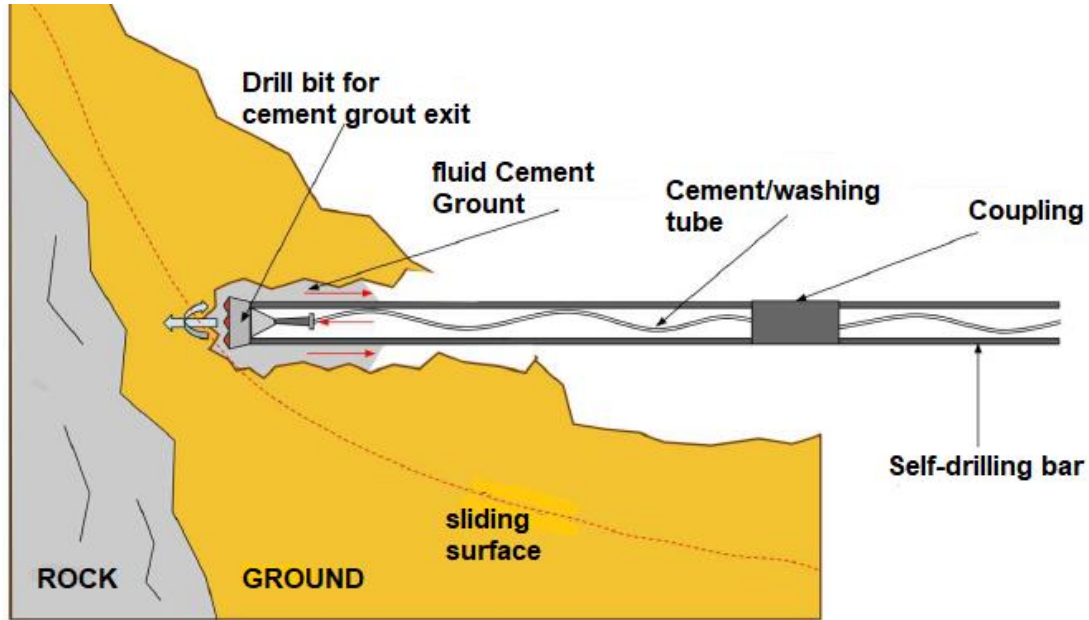
⁽⁵⁾ SIRIVE srl, Cornedo Vicentino

Contents

1. Passive Composite Anchor
2. Examples and installations
3. Advantages & Open questions
4. DFOS for real time control
5. First experiences in lab
6. Giucano Landslide
7. Fantoni landslide: long terms monitoring
8. Pull-out test



Passive Composite Anchor



Passive Composite Anchor



Hollow Bar (eventually galvanized) $T_{\max} = 1680 \text{ kN}$



Bar with strands $T_{\max} = 3150 \text{ kN}$



Drilling bits



Accessories (drilling bits, sleeves, clamping nuts)

Some installations

Val di Maso landslide
(Valli del Pasubio, VI)



Gisbenti landslide
(Valli del Pasubio, VI)

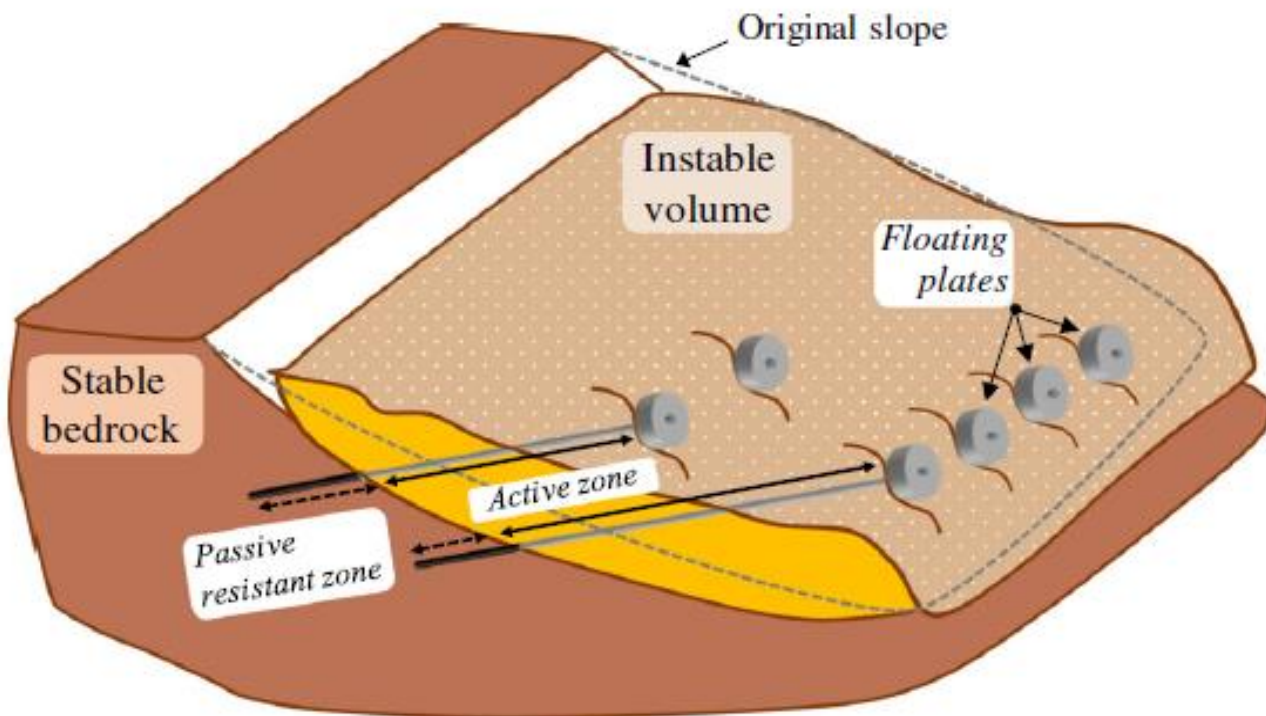


Cischele landslide
(Recoaro Terme, VI)

... and many more.....

Advantages

- ❑ **Passive anchors: not pre-tensioned** (no maintenance). Traction develops due to the friction along the soil-bar interface, as the landslide move
- ❑ **Self-drilling installation, higher resistance with minor cost, more adaptability to the stratigraphy during installation, minor external impact**

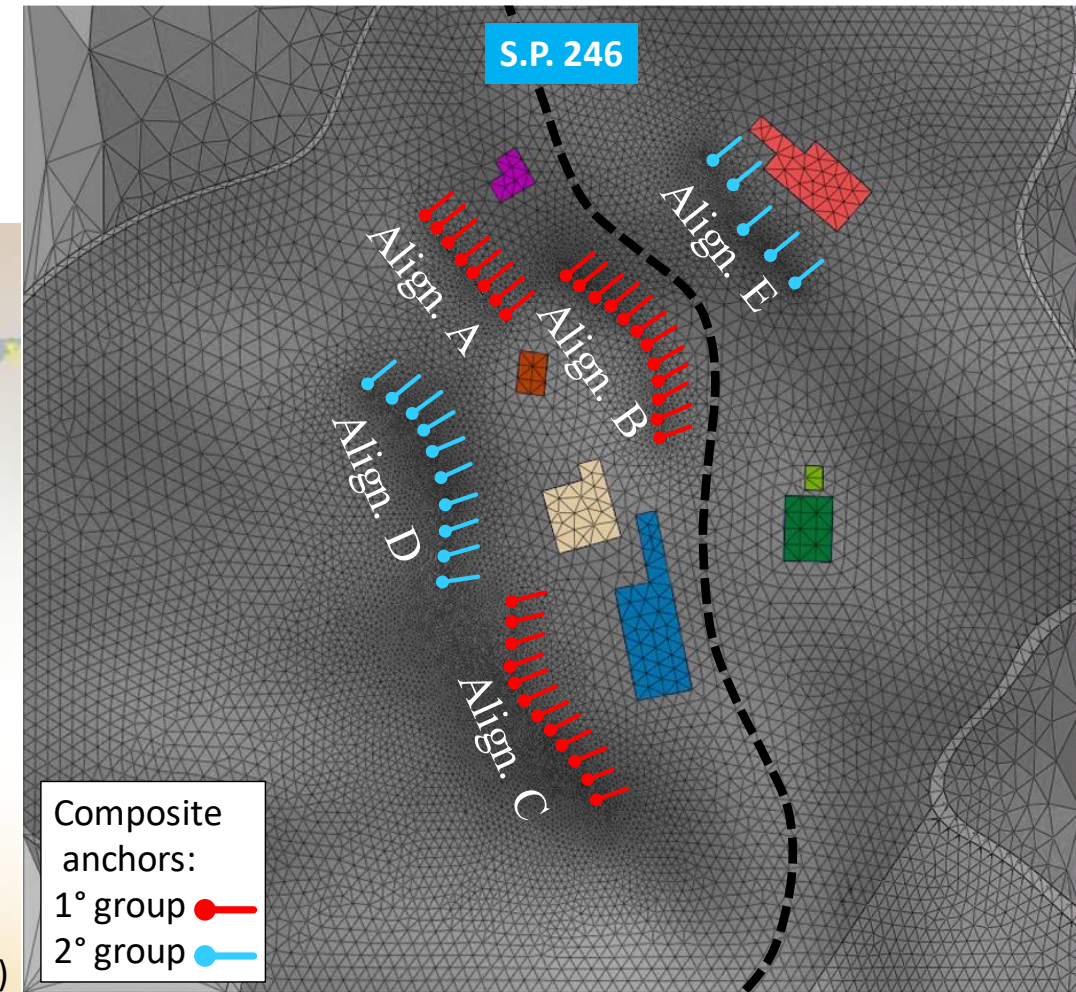
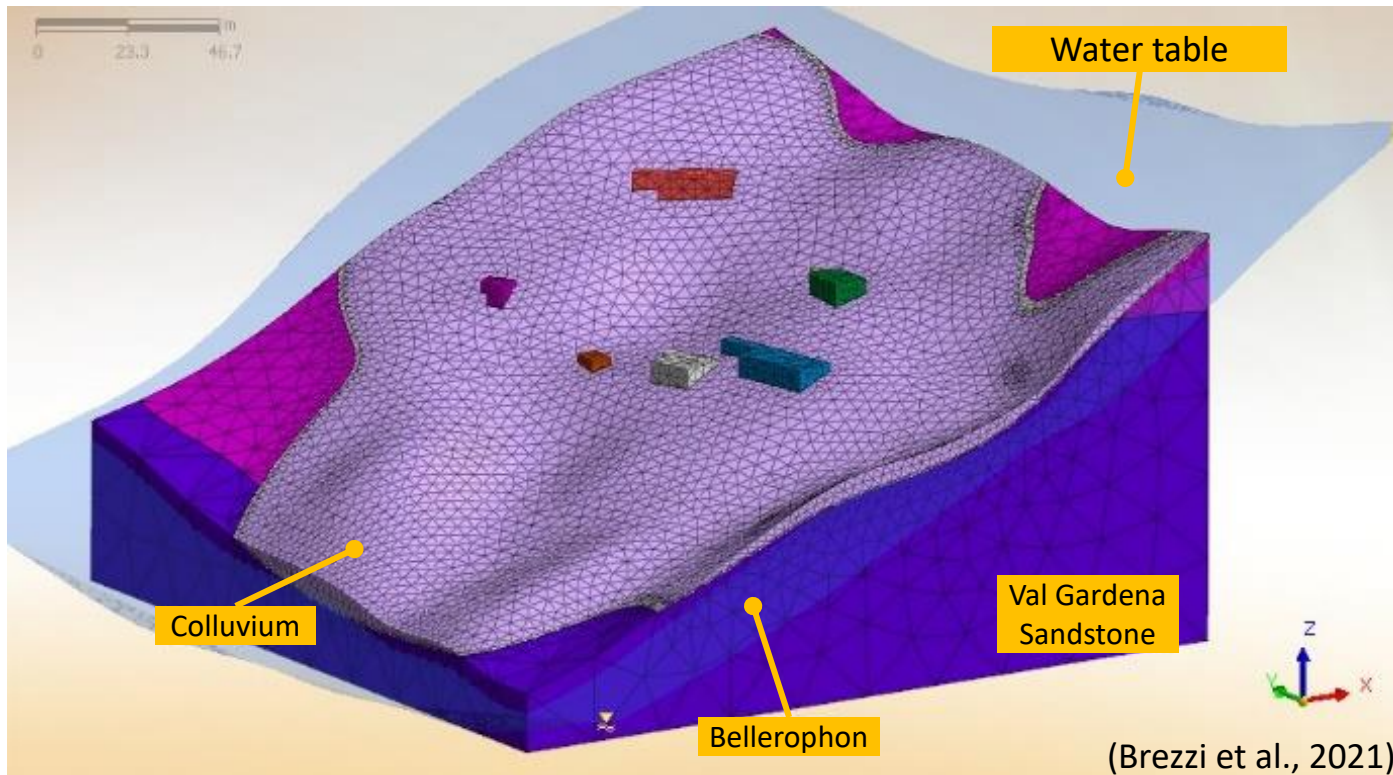


Open questions

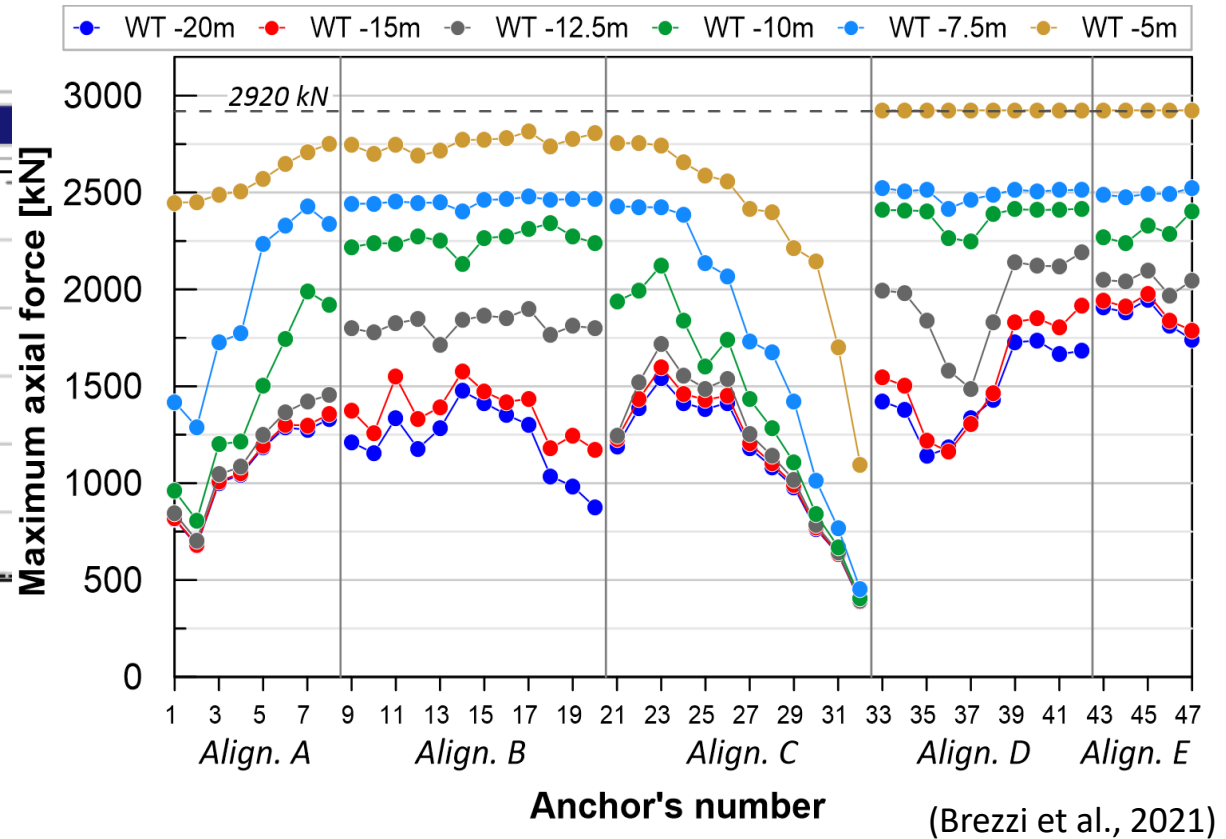
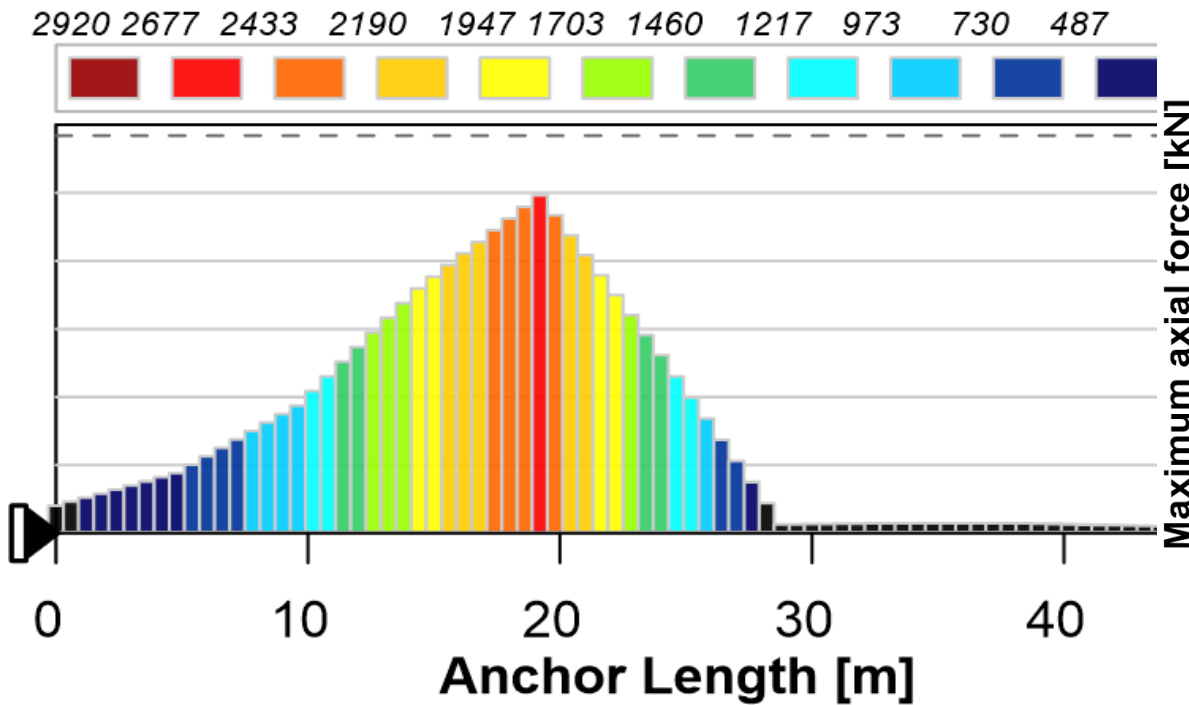
- What about the **correct coupling bar-strands**?
- Which is the **load distribution along the bar**?
- Is the **external plate large enough**?

Numerical evaluation of traction with FEM

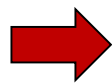
- ❑ Cischele landslide analyzed with FEM-3D and Strength Reduction Method (SRM)
- ❑ Without and with different lines of anchors
- ❑ Various water levels



Numerical evaluation of traction with FEM



Research goal



Set up of a monitoring system to measure and verify the development of load over time in the anchors

Laboratory tests

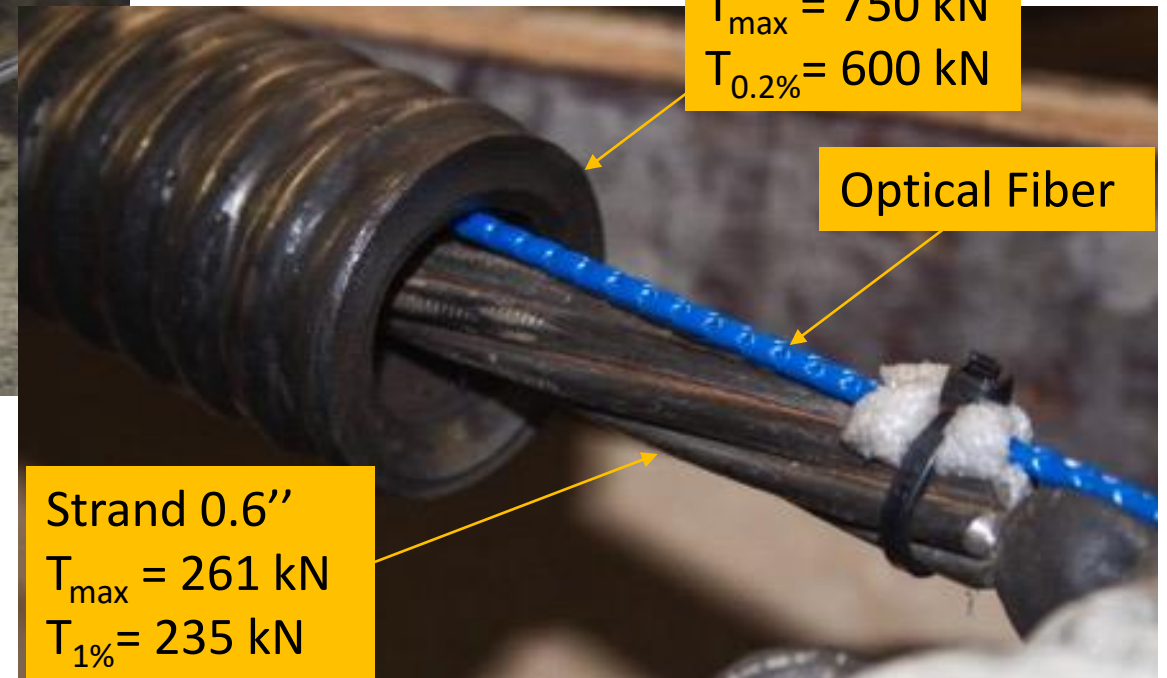
Simple bar

Bar + mortar + DFOS



Composite bar

Bar + strand + mortar + DFOS



Bar R51

$T_{\max} = 750 \text{ kN}$

$T_{0.2\%} = 600 \text{ kN}$

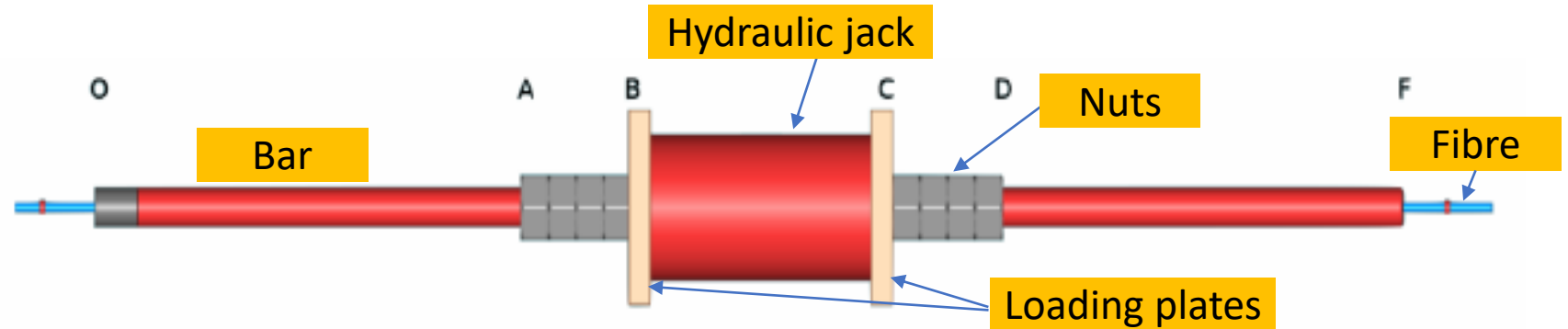
Optical Fiber

Strand 0.6''

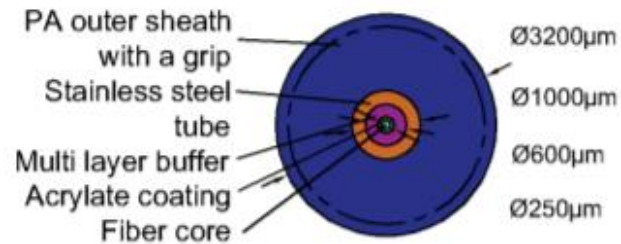
$T_{\max} = 261 \text{ kN}$

$T_{1\%} = 235 \text{ kN}$

Laboratory tests

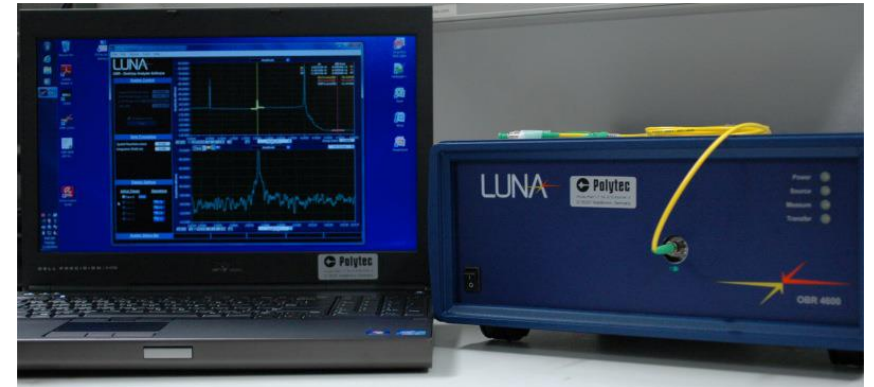


BRUSens V9 strain cable (Ø 32mm)



- Strain range up to 1% (10000 µε)
- Attenuation < 0.5 dB/km for a wavelength of 1550 nm
- Max tensile strength at installation: 260 N

Luna OBR4600 (Rayleigh-OFDR)



- One ended configuration
- Sensing range: 70m – 2km
- Accuracy: 1µε / 0.1°C
- Spatial resolution: 10µm – 1mm

Laboratory tests

- Tensile traction:

$$Q(s) = (EA)_{eq} \varepsilon$$

with $(EA)_{eq}$ = equivalent stiffness of composite bar

$$(EA)_{eq} \approx E_b A_b + n \cdot E_s A_s$$

where:

$$E_b = E_s = 201 \text{ GPa}$$

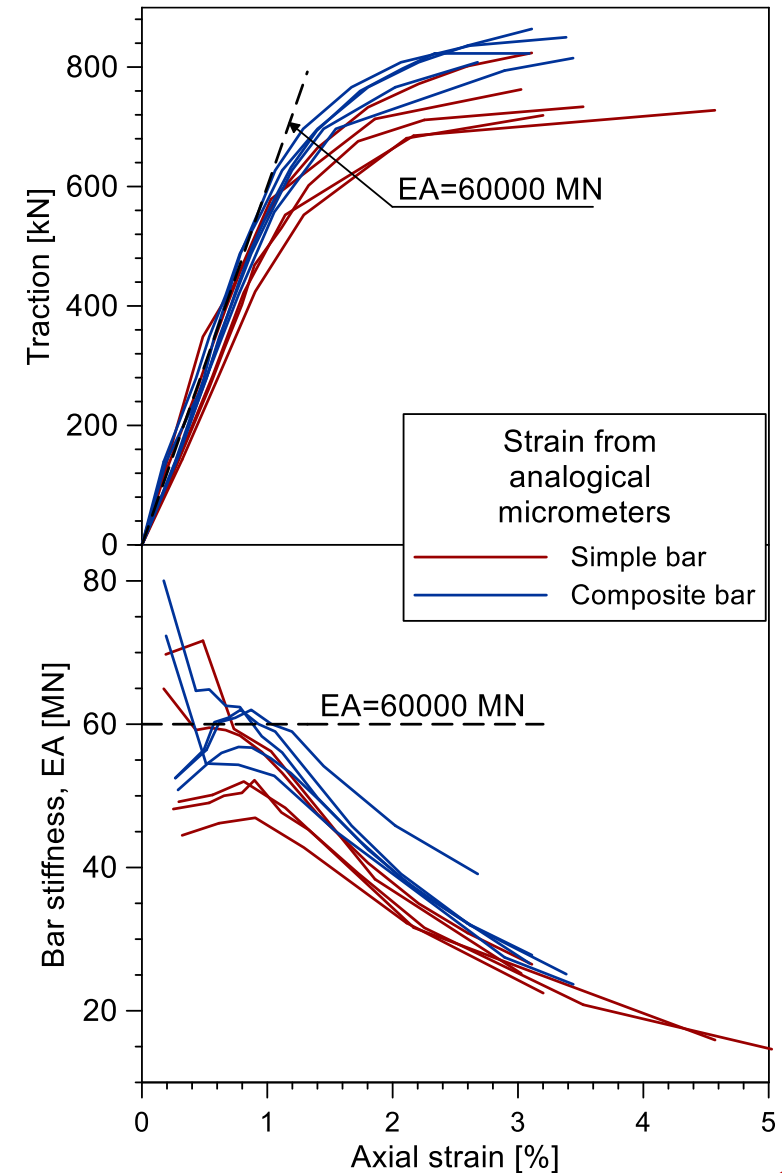
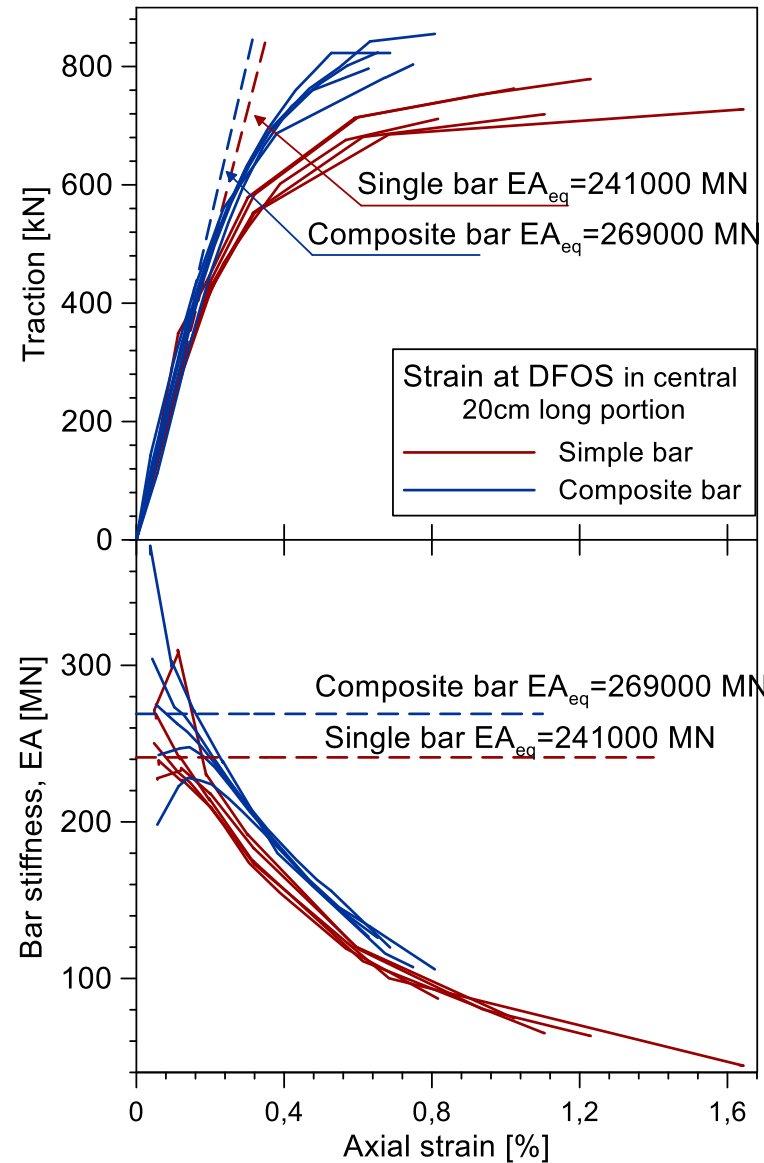
$$A_b = 1200 \text{ mm}^2$$

$$A_s = 140 \text{ mm}^2$$

$$n = 1$$

$$(EA)_{eq} = 241000 \text{ MN Single bar}$$

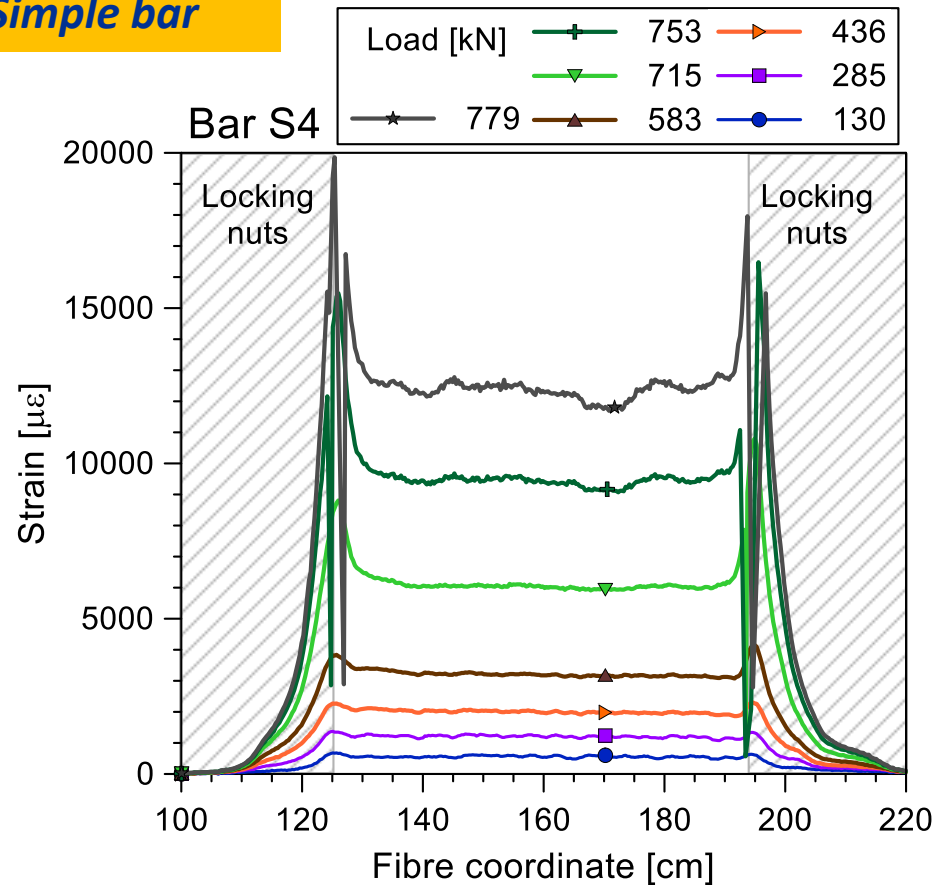
$$(EA)_{eq} = 269000 \text{ MN Composite bar}$$



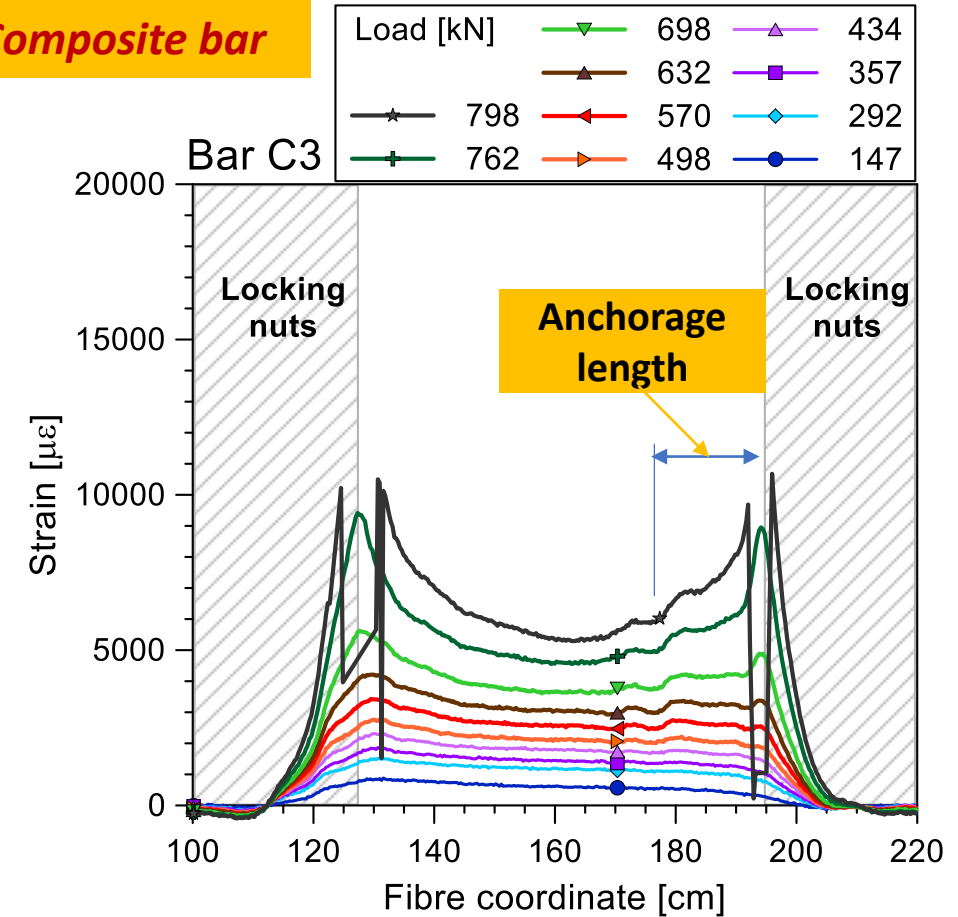
(Cola et al., 2019)

Misure in laboratorio

Simple bar



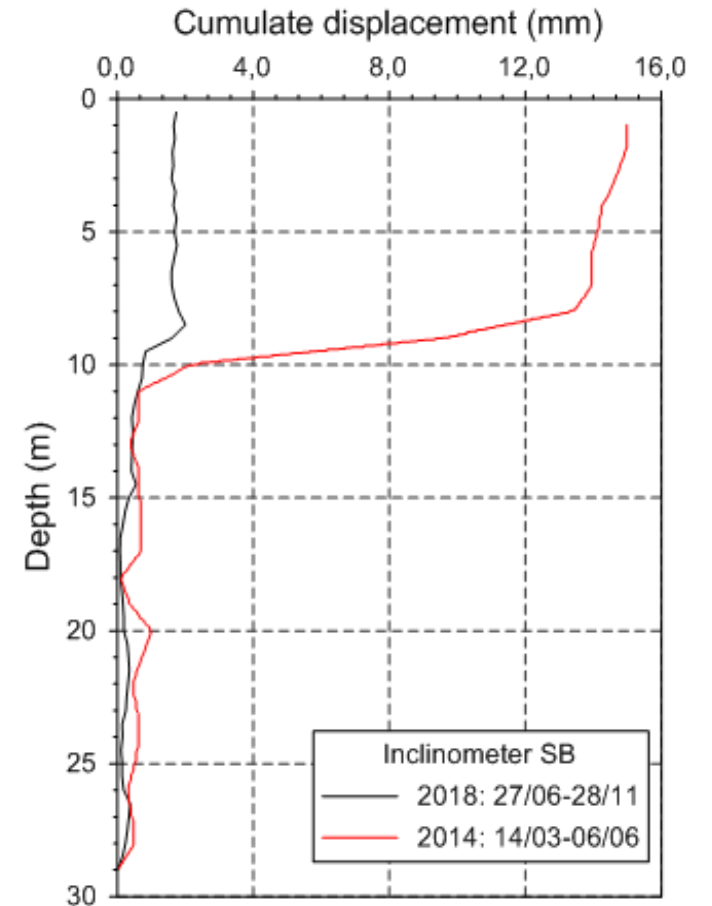
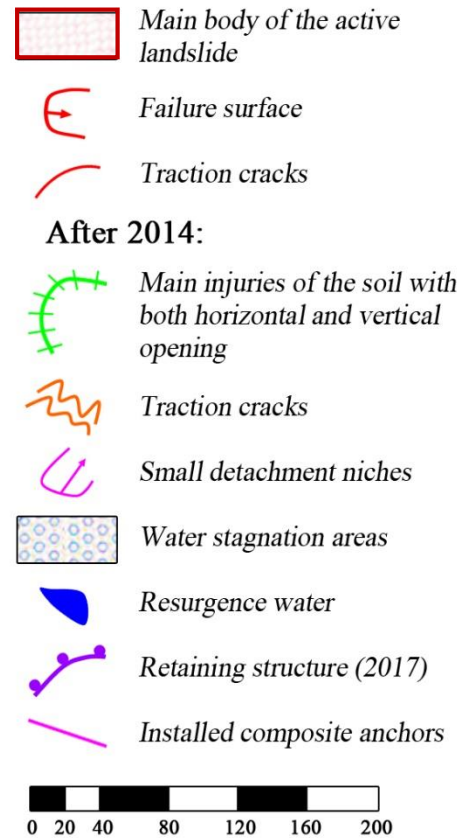
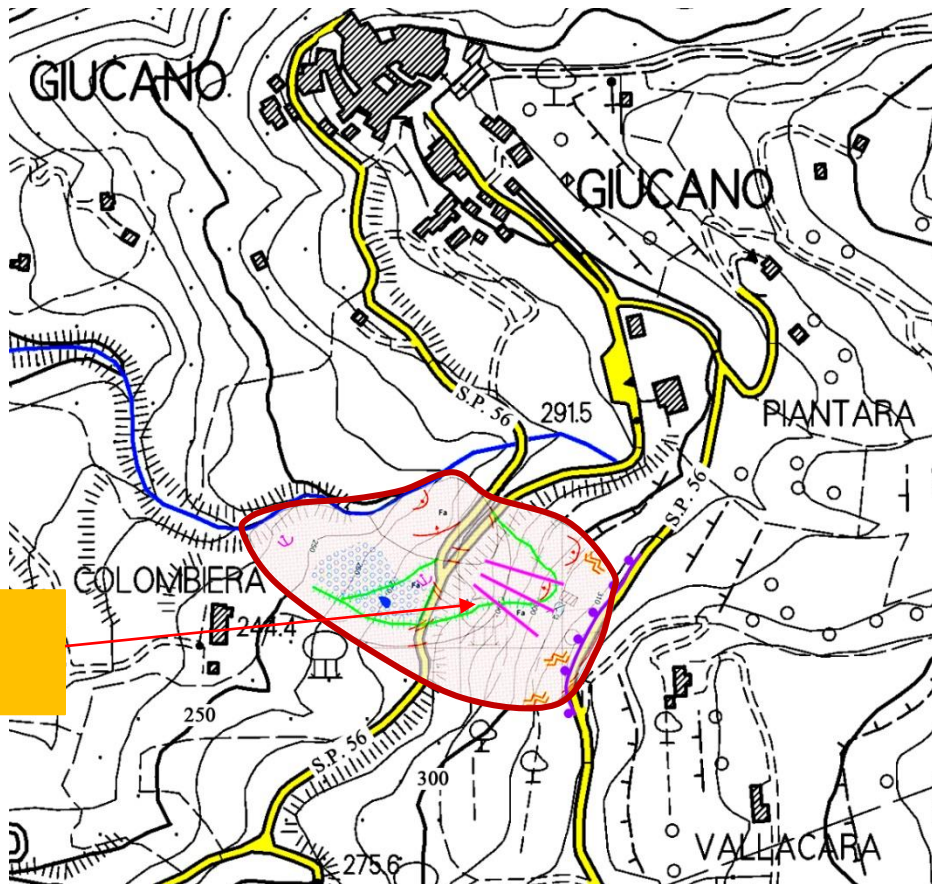
Composite bar



(Cola et al., 2019)

2° in-situ test at Giucano landslide

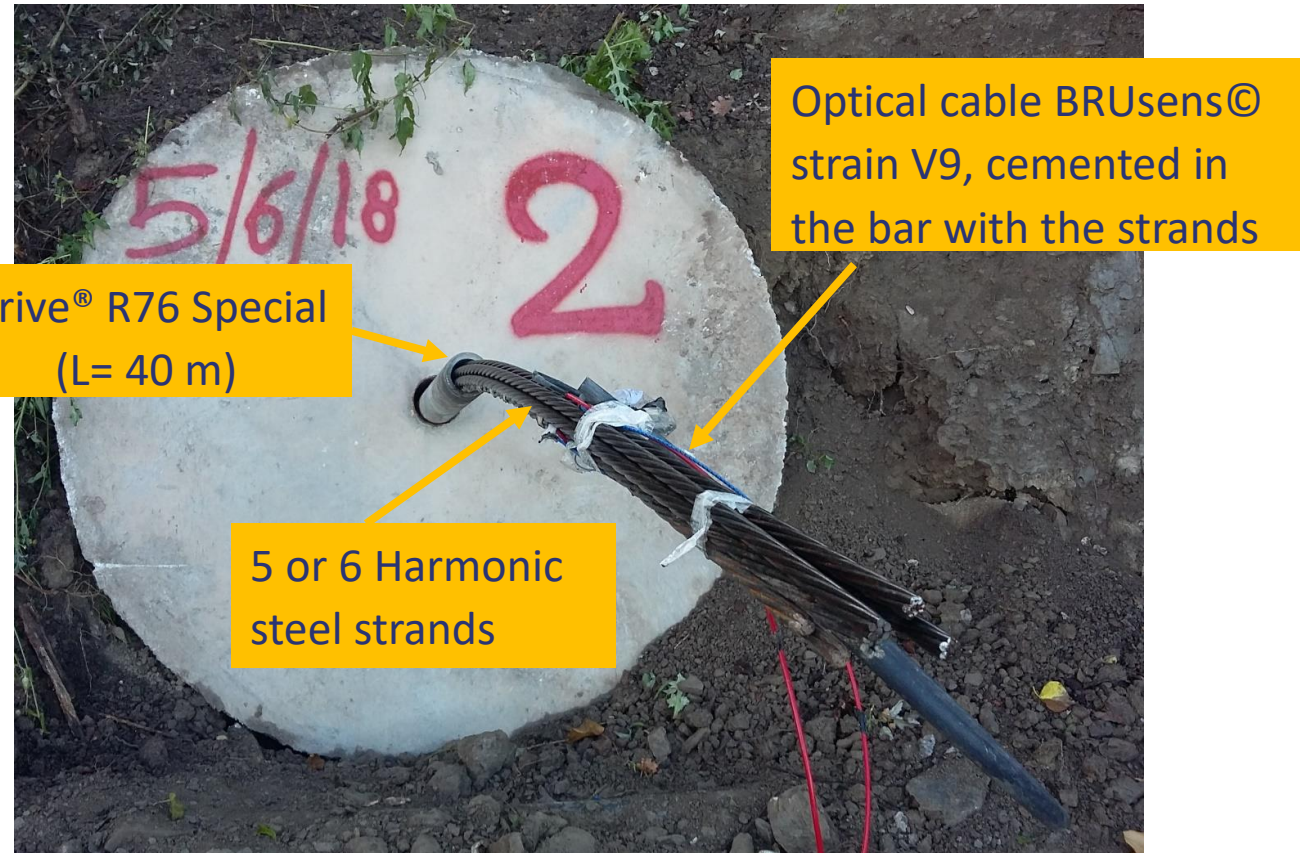
- Giucano (Fosdinovo, Toscana)
- Activated in 2013, not stabilized even if some interventions were performed in the upper border



2° in-situ test at Giucano landslide



- 3 Bar Sirive® R76 Special ($T_{\max} = 1159$ kN, 40 m-long)
- 5 (bar n.1) or 6 (bars n.2 and 3) 0.6" strands ($T_{\max} = 240$ kN)



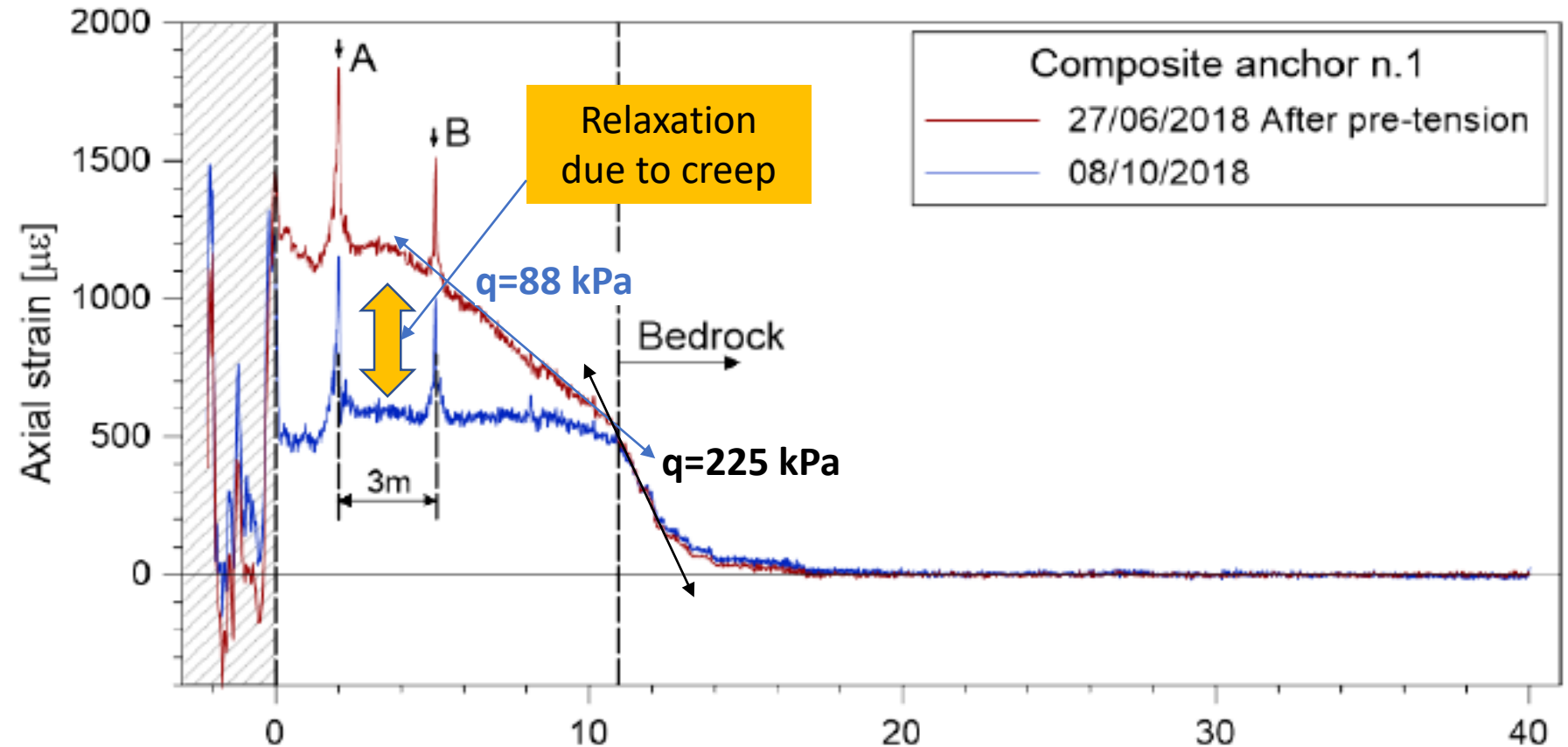
2° in-situ test at Giucano landslide

- Mobilized lateral resistance:

$$q = \pi D_c \frac{dQ}{ds} \approx \pi D_c \frac{\Delta Q}{\Delta s}$$

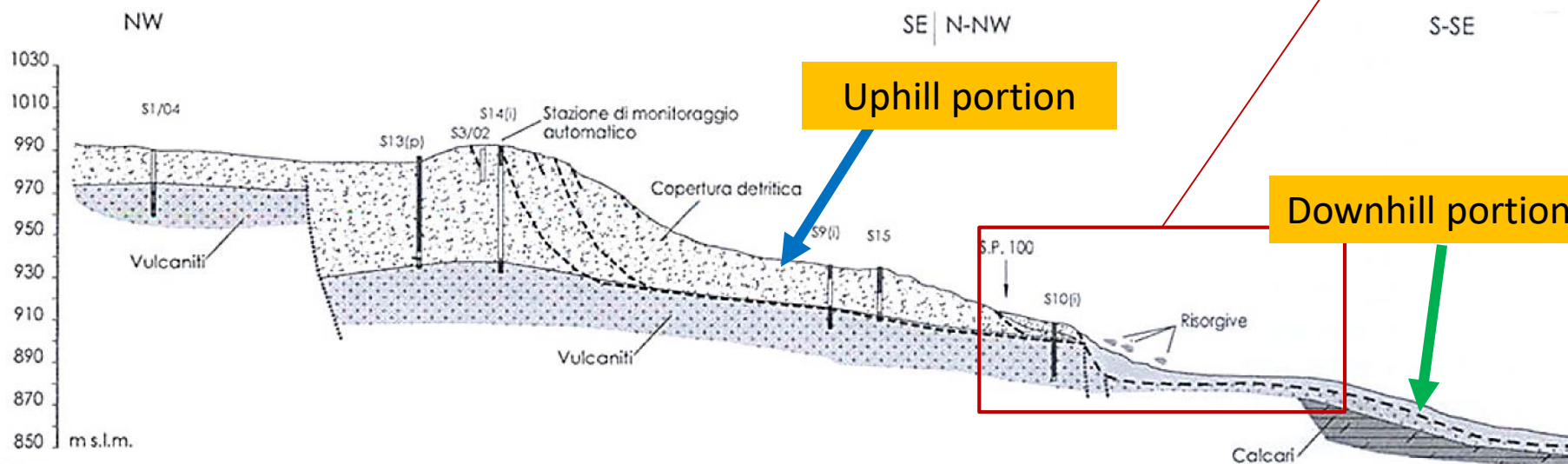
This can be calculated in the portions with constant slope.

Each anchor is tensioned after the mortar has hardened. Strain is measured after the hardening and 3 months later



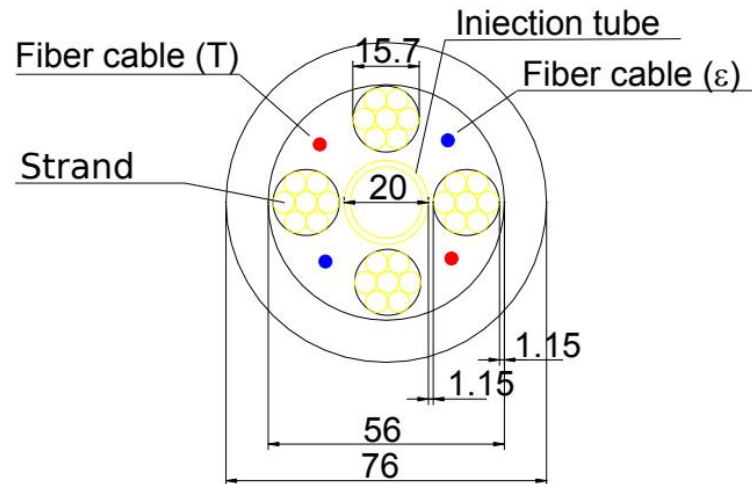
2° in-situ test at Fantoni landslide

- ❑ **Site:** Contrada Fantoni, Recoaro Mille (VI)
- ❑ **Landslide estension:** 57.000 m²
- ❑ **2 portions:** uphill a roto-translational movement, downhill a slow creeping slope
- ❑ **Material:** coarse colluvioum above Vulcanic rock or Limestone



2° in-situ test at Fantoni landslide

- ❑ Optical cable BRUsens© strain V9, installed in double mode
- ❑ Optical cable BRUsens© temperature only in anchor 5



7 composite anchors installed in November-December '20

- ❑ Bar SIRIVE® S76 driven up to 5-6 m in the stable bedrock
- ❑ 4 strands
- ❑ Different lengths and inclinations

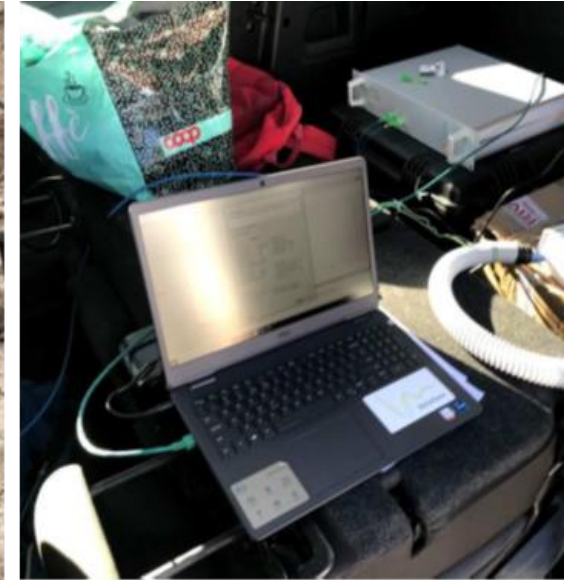


2° in-situ test at Fantoni landslide

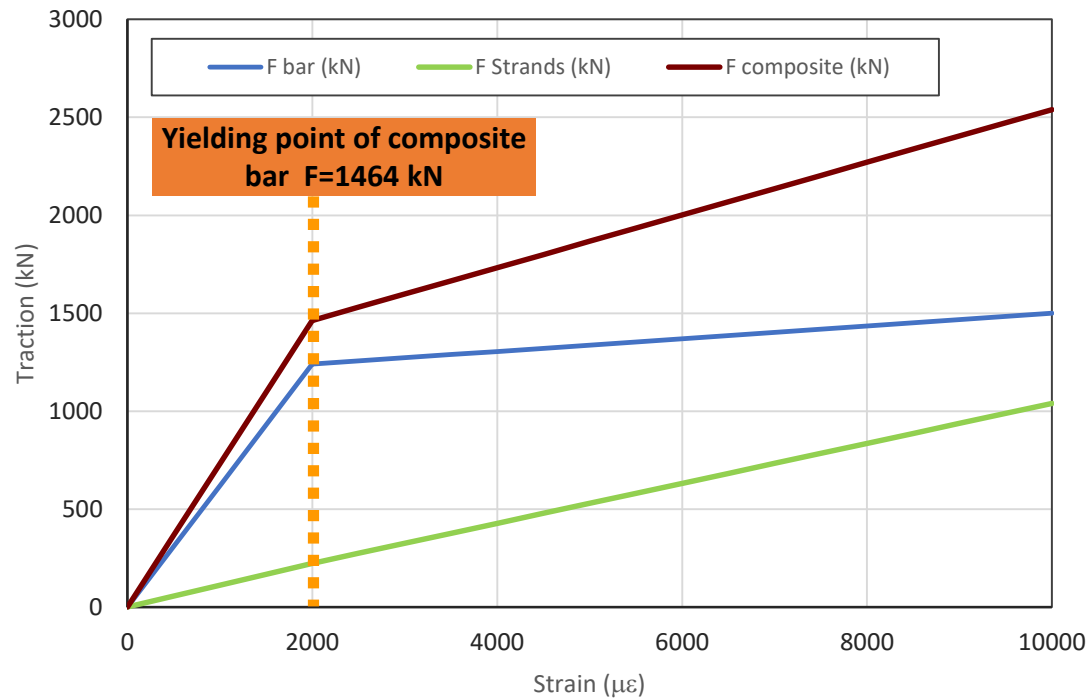
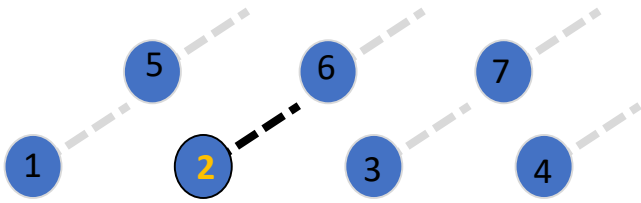


2° in-situ test at Fantoni landslide

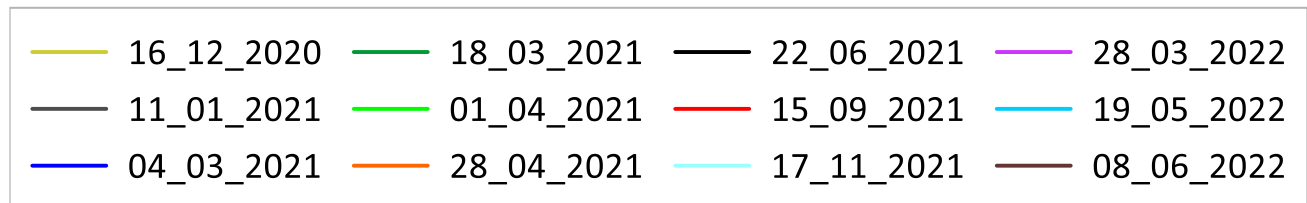
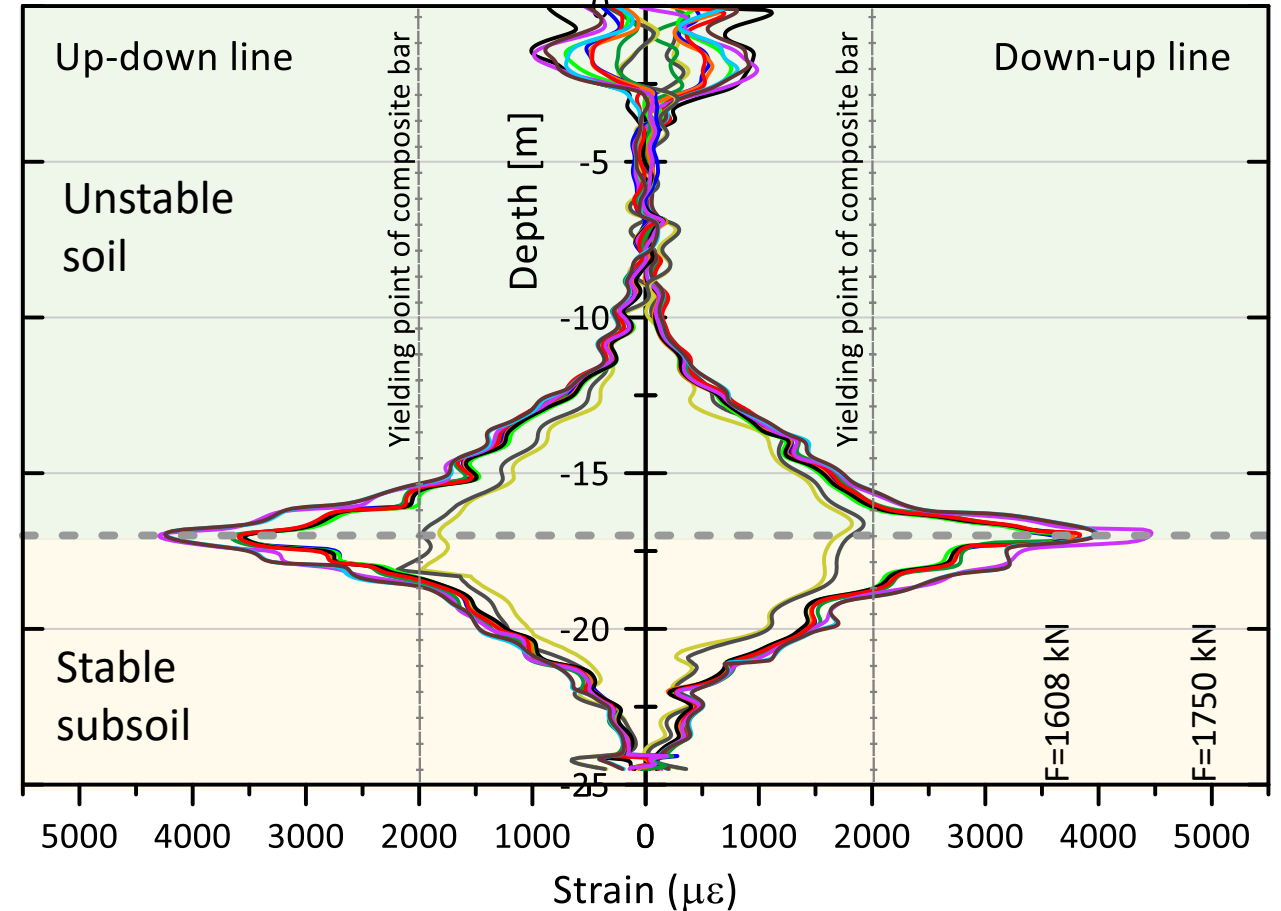
- ❑ DFOS installed on 01/12/2020
- ❑ Brillouin Optical Frequency Domain Analyzer (BOFDA) from fibrisTerre (Germany) with:
 - ❑ maximum sampling: 5
 - ❑ spatial resolution: 20 cm
 - ❑ strain measurement accuracy: $2 \mu\epsilon$
 - ❑ temperature measurement accuracy: 0.1°C
- ❑ Reference reading at 02/12/2020
- ❑ 10 readings with time from December 2020 to June 2022



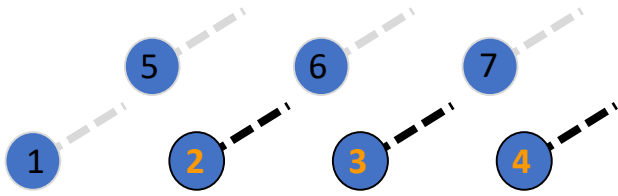
2° in-situ test at Fantoni landslide



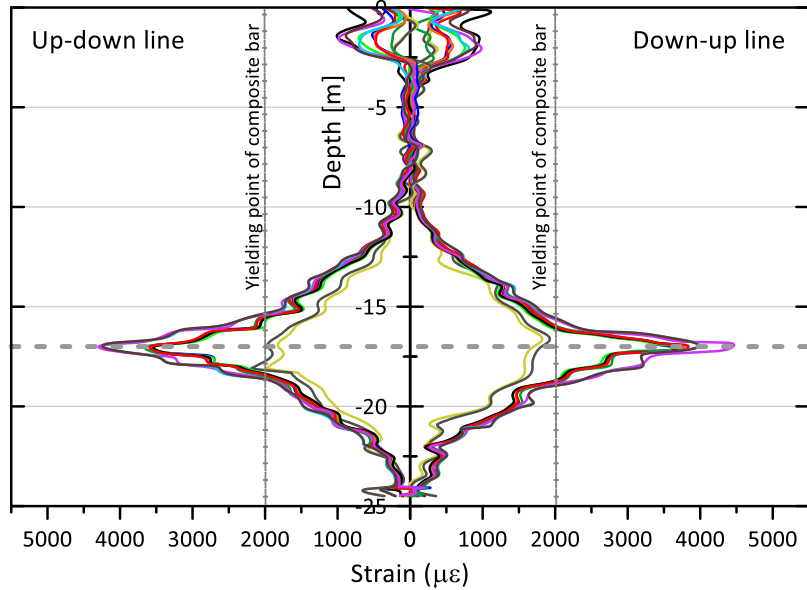
Anchor 2



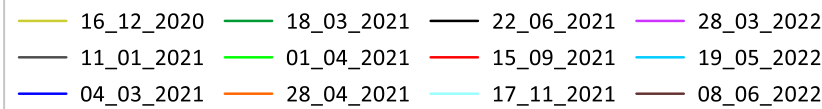
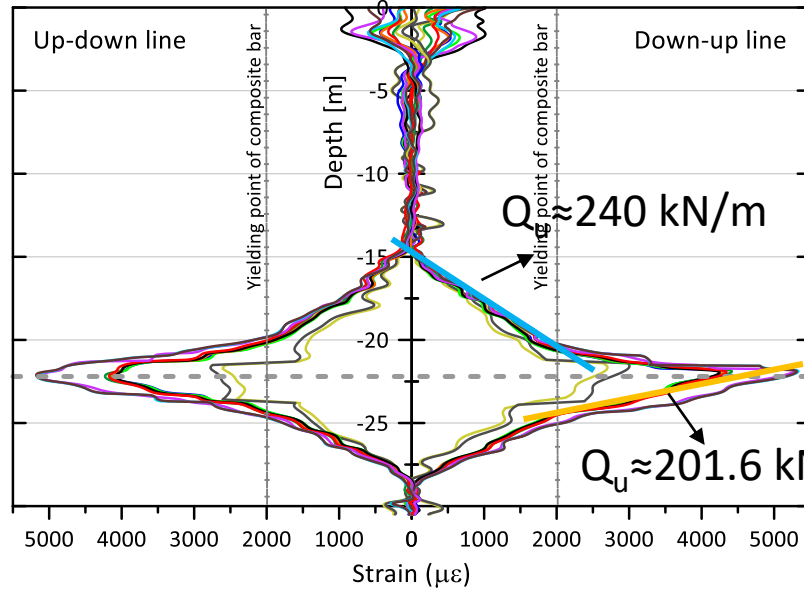
Strain distribution over time



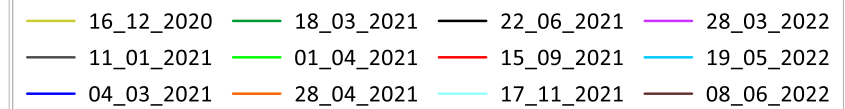
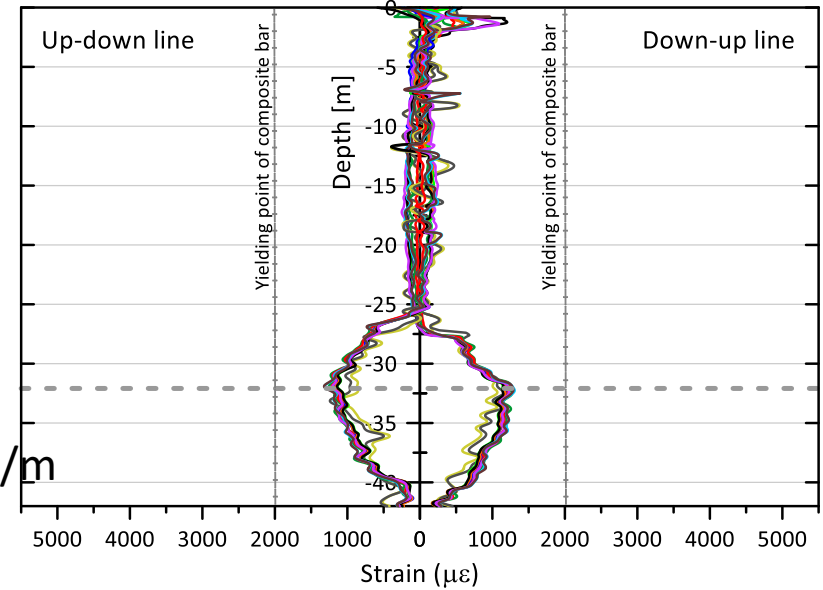
Anchor 2



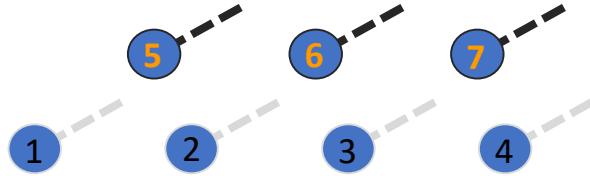
Anchor 3



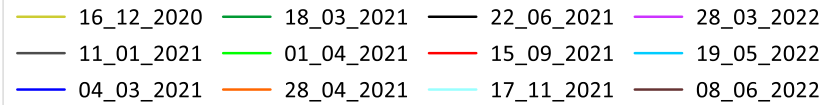
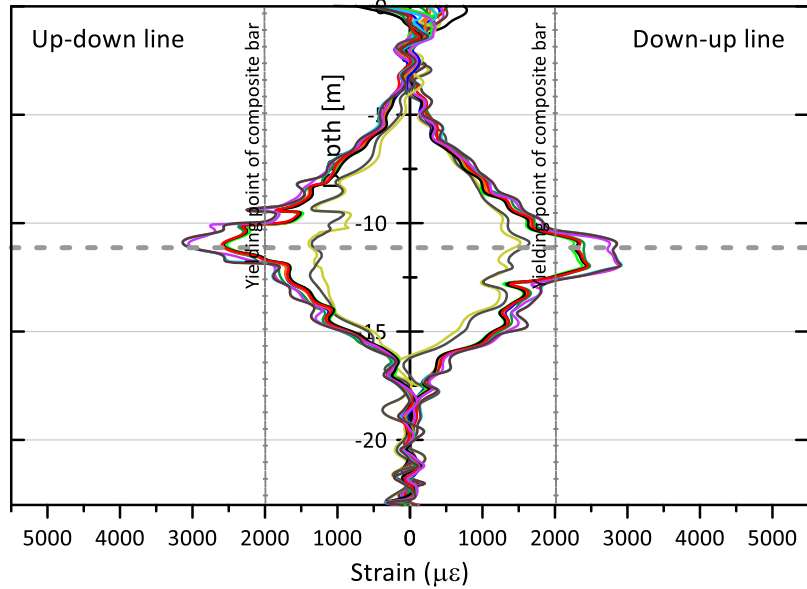
Anchor 4



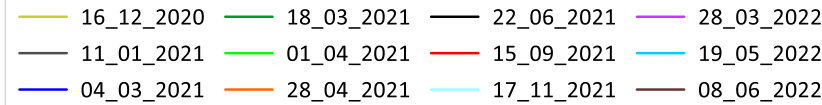
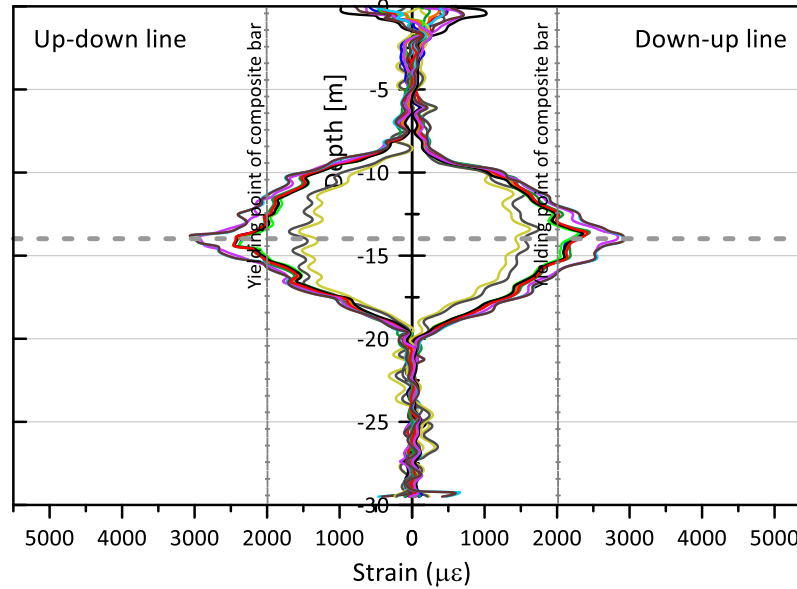
Strain distribution over time



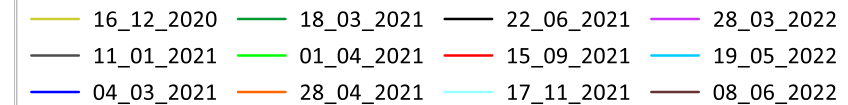
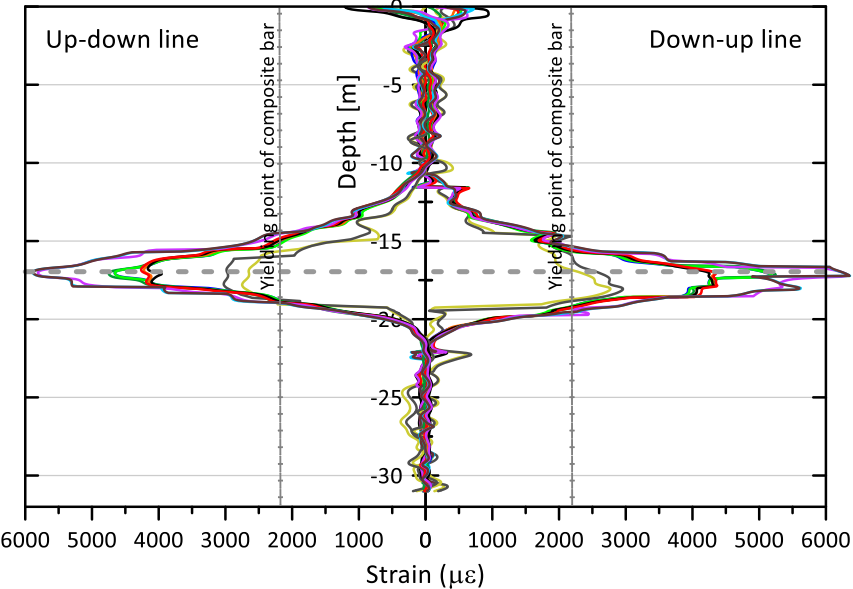
Anchor 5



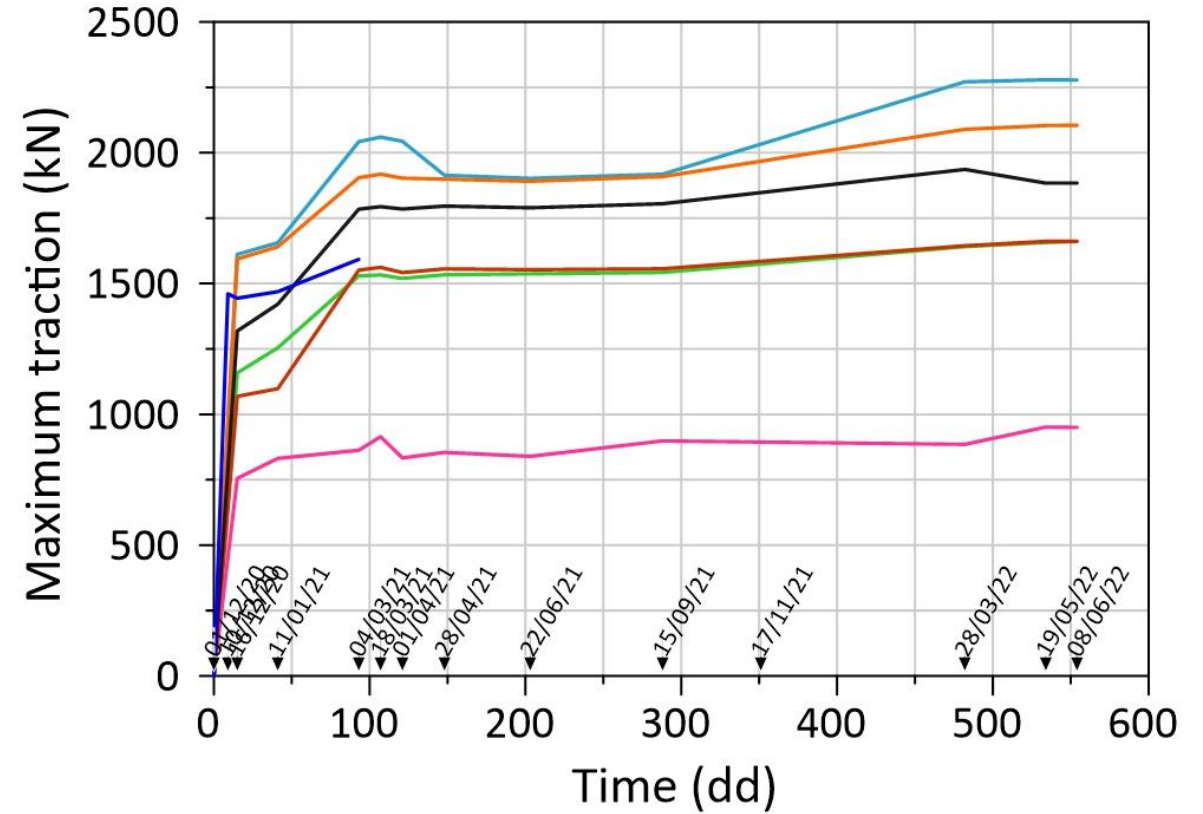
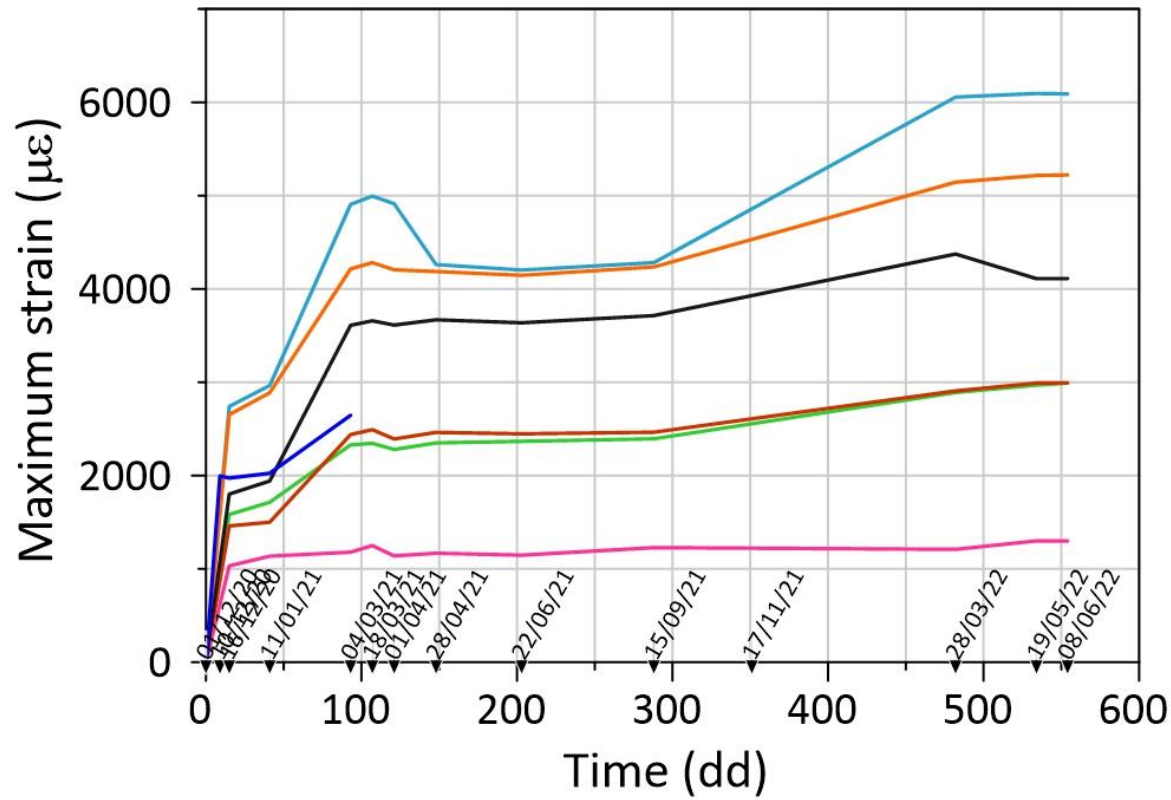
Anchor 6



Anchor 7



Strain and traction over time



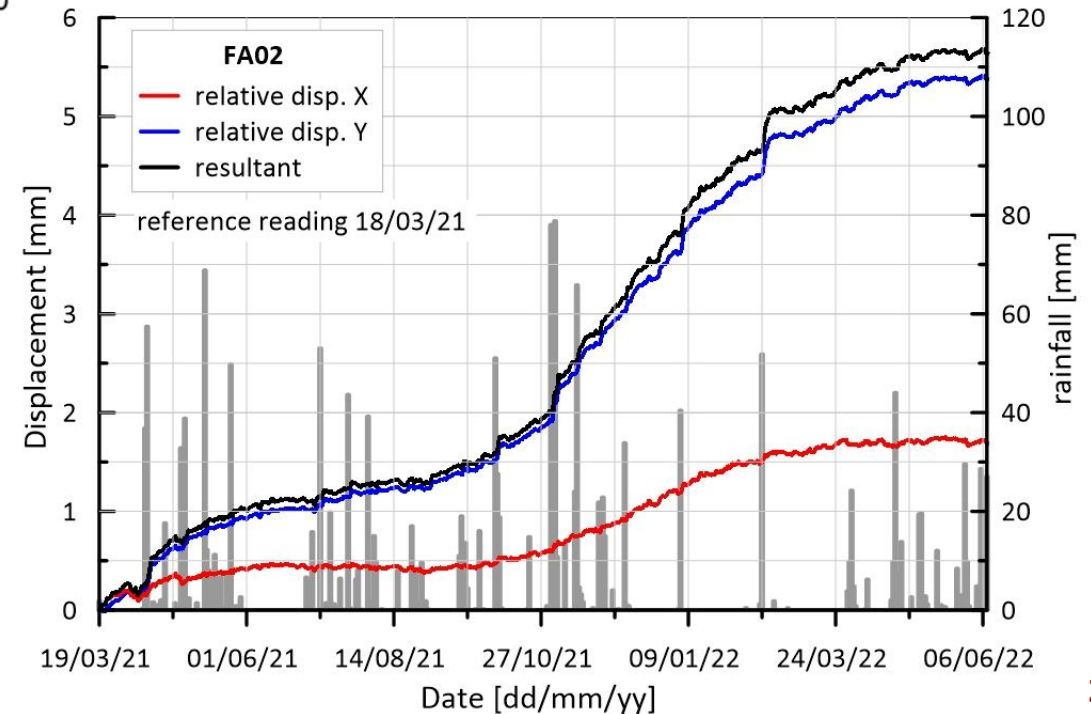
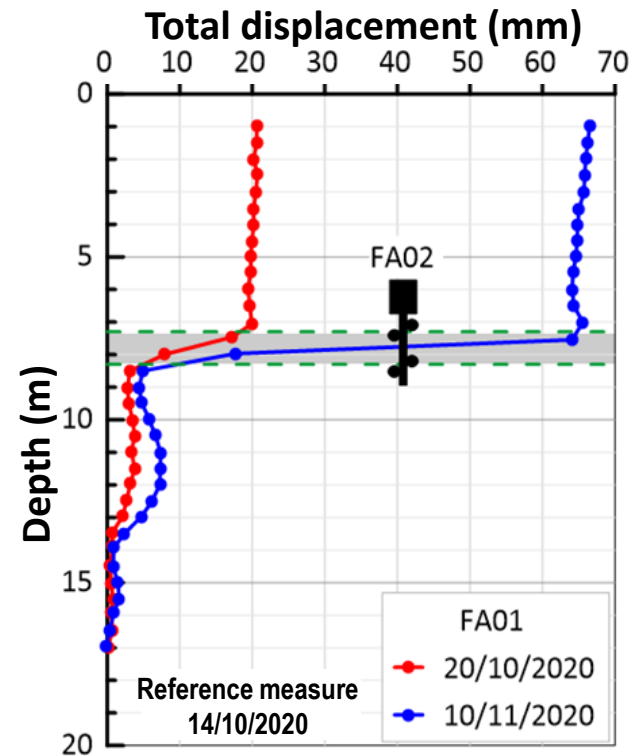
Anchor



2° in-situ test at Fantoni landslide

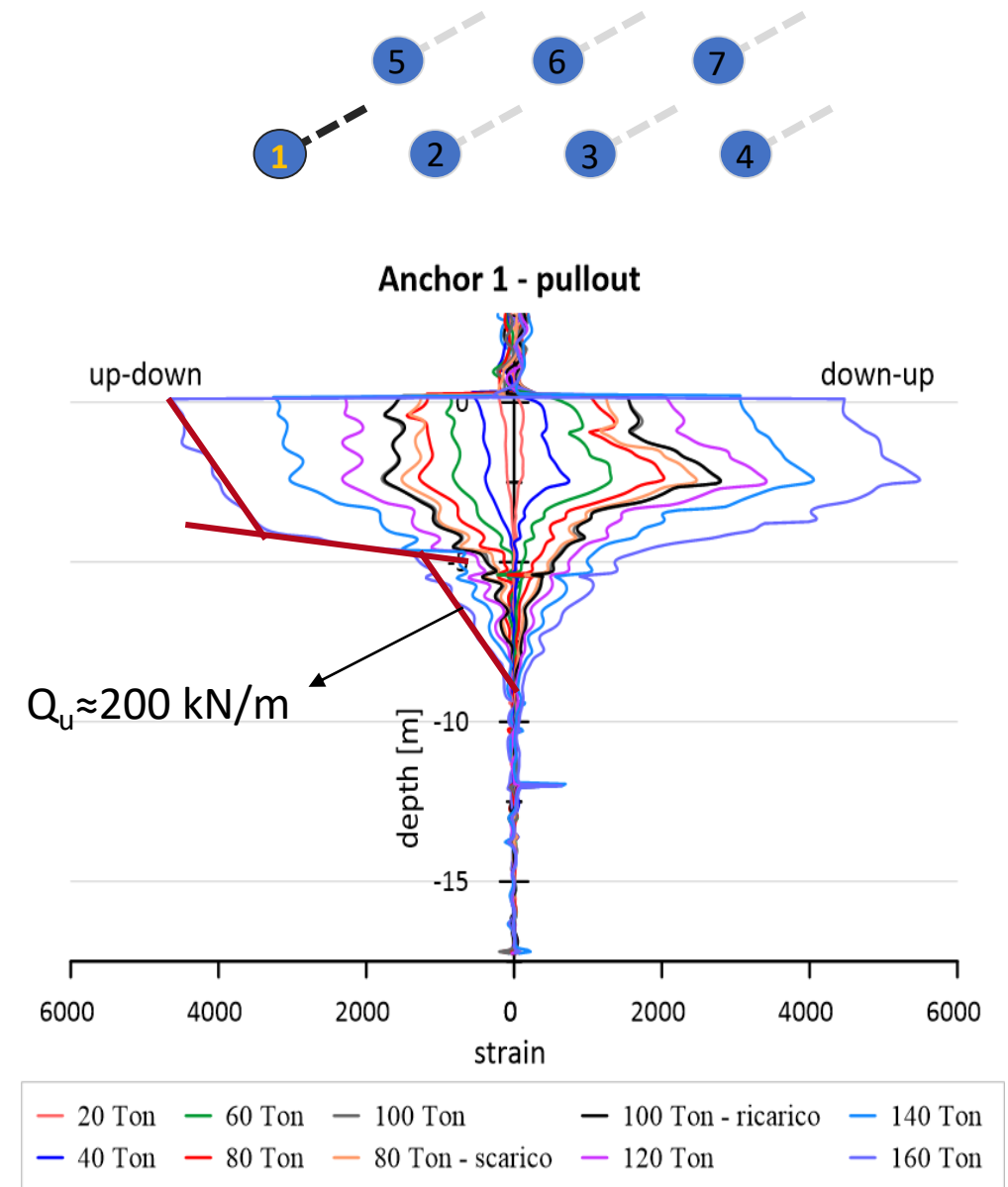
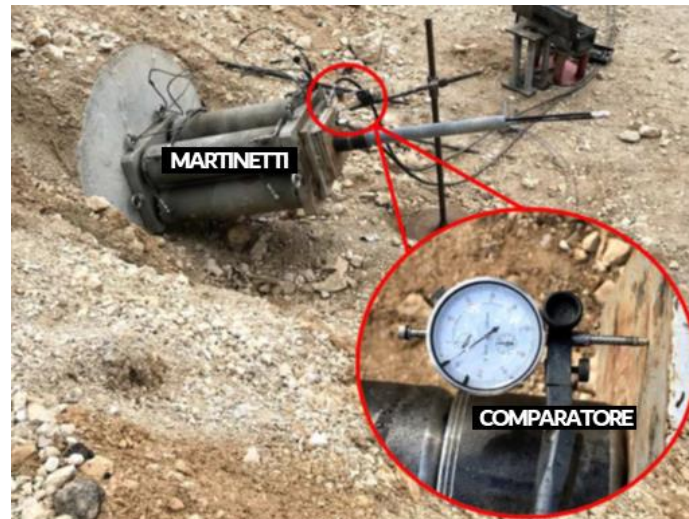
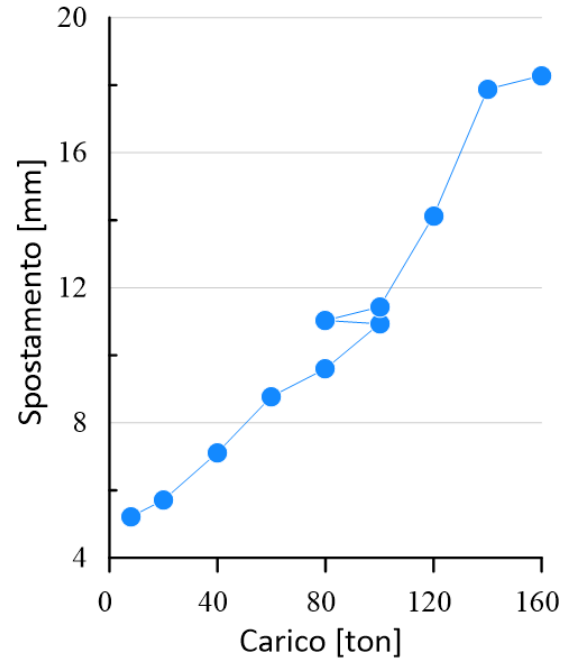
- ❑ Manual inclinometer **FA01**
installed pre-anchors
 - ❑ 14/10/2020 - 10/11/2020
con 6.7 cm in 27 gg ($v=2.5 \text{ mm/g}$)
- ❑ Fixed inclinometer **FA02**
installed post-anchors
 - ❑ March – June 2021:
1,1 mm in 3 months
($v=0,012 \text{ mm/g}$)
 - ❑ October 2021 – March 2022:
3,8 mm in 6 months
($v=0,021 \text{ mm/g}$)

➔ Interrupted



Pull-out test

- ❑ Failure at 160 ton \approx 1600 kN due to lost of contact between mortar and bar in the external portion
- ❑ Not all the bar was involved in load
- ❑ Some bending moment in the external portion
- ❑ Information about the unitary pull-out resistance



Final remarks of DFOS application

- In this application DFOS demonstrate to be the most proper technique for studying the behavior of composite anchors in laboratory but also in site
- They permit to study the interaction of the bar with the inner strands and the presence of bending moments or local discontinuities
- In the in-situ measurements, they give the possibility to:
 - measure the distribution of axial force along the bar
 - individuate the position of the sliding surface
 - understand the interaction between composite anchor and soil
 - evaluate its behavior over time
 - evaluate the effective contribution given for the landslide stabilization
- In the pull-out test, they demonstrate that the test procedure needs to be modified if the aim is to characterize the resistance of the deep stable soil

Thanks you for the attention!

Acknowledgements:

- **Dalla Gassa srl:** Gaetano Dalla Gassa, Alberto Bisson and all the staff
- **UNIPD:** Lorenzo Brezzi, Simonetta Cola, Paolo Simonini, Fabio Gabrieli, Nicola Fabbian, Antonio Pol, Francine Tchamaleu Pangop, Fabio Schiavon, Zeno Lucchese
- **CNR-IRPI Padova:** Luca Schenato, Giacomo Tedesco, Gianluca Marcato
- **EPC:** Giordano Munaretto, Federico Carollo
- **M3E:** Nicolò Spiezia
- **PROV. VICENZA:** Massimo Lovison
- **GEOASSET srl:** Massimo Moracchiolo